

Figure 61 Vertical displacements along LUAS light railway (induced by wall installation and excavation)



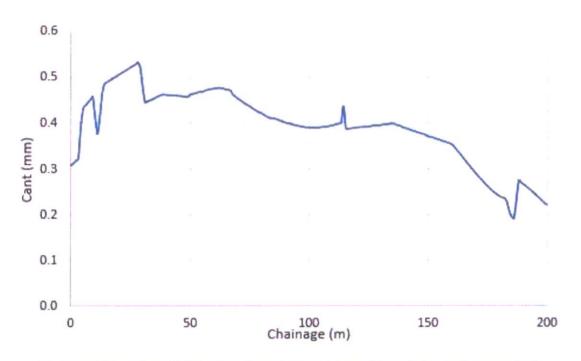


Figure 62 Cant along LUAS light railway (induced by wall installation and excavation)

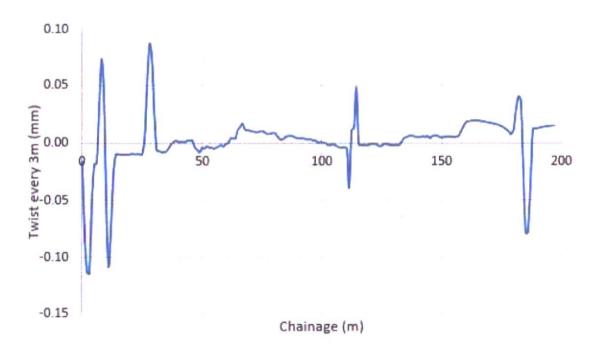


Figure 63 Twist (at 3m spacing) along LUAS light railway (induced by wall installation and excavation)



### 7.7 Assessment of Local Sewers

A number of utilities are present in proximity of the proposed development site. These utilities comprise sewers of varying size and materials. Details relating to the assessed utilities are shown in Table 6. The location of the assets in relation to the site and the reference names are shown in Figure 63.

All sewers are assumed to be circular in cross section (for the sake of conservatism, the maximum diameter has been considered in the analyses). In general, lining thicknesses have been assumed for the utilities where no information is available, as shown in Table 5. The assumptions should be confirmed/revised as appropriate during design development.

The impact assessment on the sewers has been undertaken focusing on the key deformation mechanisms and performance criteria applicable to the utility types noted, including tensile strains (induced by axial elongation and bending mechanisms), joint rotation and pull-out. Limit criteria are based on typical figures representative of the types of utilities under consideration, generally based on Thames Water's guidance for developers. The adopted values are summarised in Tables 7 and 8.

In the assessment, a neutral axis position at the edge of the pipes has been considered, in order to evaluate tensile strains arising as a result of bending mechanisms. The axial strains (i.e. tensile strains due to pipe extension) that the concrete/vitrified clay sewers are subjected to have been factored to 20% to model the soil/pipe interface.

Table 6 Sewer names and dimensions

Asset Name	Material	Internal Diameter (mm)	Wall Thickness (mm)
Sewer 1	Masonry	1750	300
Sewer 1b	Masonry	2200	300
Sewer 2a	Masonry	810	300
Sewer 2b	Masonry	810	300
Sewer 2c	Masonry	810	300
Sewer 3	Vitrified Clay	300	50
Sewer 4	Vitrified Clay	300	50
Sewer 5	Unreinforced concrete	300	50
Sewer 6	Vitrified Clay	300	50
Sewer 7	Vitrified Clay	225	50
Sewer 8	Masonry	810	300



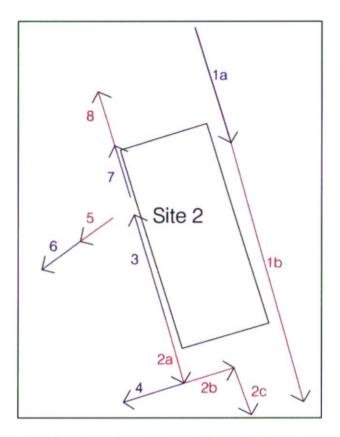


Figure 64 Sewers locations (arrows indicate positive distance direction along utility assumed in assessment)

An indicative view of the Xdisp models, including the excavation/retaining system installation areas, is presented in Figure 62.



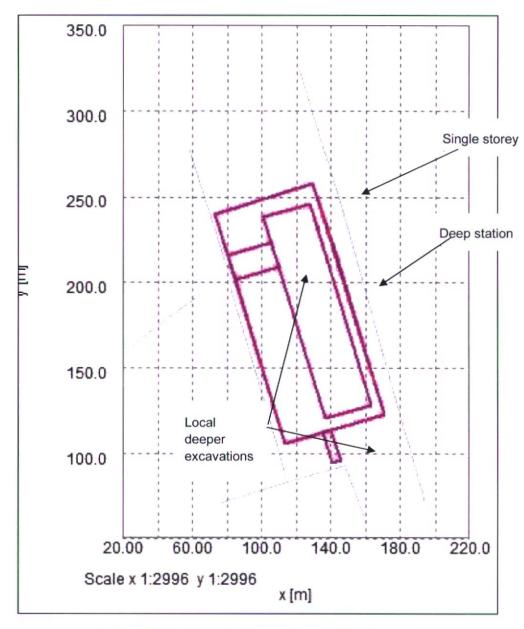


Figure 65 Indicative view of the Xdisp model (plan view)

### 7.8 Impact Assessment

Tables 6 and 7 below summarise the results of the assessment.

The output *greenfield* deflected profiles for multiple utilities include a series of sharp discontinuities which induce apparent exceedances of the limit strain criteria. These discontinuities are a consequence of the relatively simplistic analytical approach adopted and are considered unrealistic. Therefore, a smoothing exercise has been undertaken, fitting polynomial relationship curves to the displacement profiles affected. The process has been carried out using an automated algorithm which



eliminates the abrupt displacement changes. The smoothed profiles have subsequently been imported in a new Xdisp file in which the damage assessment is carried out.

The results indicate that maximum tensile strains are within the allowable limits with the exception of Sewer 5. Plot of displacements and strains at the various stages are presented in Figures 65 to 108.

Table 7 Sewers impact assessment summary - demolition

Asset	Material	Internal Diamet er (mm)	Maximu m Tensile Strain ( με)	Tensil e Strain Limit (με)	Maximum Compressi ve Strain (με)	Compressi ve Strain Limit (με)	Maximu m Rotation	Rotatio n Limit	Maximu m Pull- out(mm)	Pull- out Limi t (mm
Sewer 1a	Masonr y	1750	28	500	-	-	-	-	-	-
Sewer 1b	Masonr y	2200	18	500	-		-	-	-	-
Sewer 2a	Masonr y	810	189	500	-	-	-	-	-	-
Sewer 2b	Masonr y	810	0	500	-	-		-		-
Sewer 2c	Masonr y	810	0	500	-	-	-	-	-	-
Sewer 3	Vitrified Clay	300	74	80	37	400	0.0	2.0	0.0	3.0
Sewer 4	Vitrified Clay	300	0	80	0	400	0.0	2.0	0.0	3.0
Sewer 5	Concret	300	0	40	0	400	0.0	2.0	0.0	3.0
Sewer 6	Vitrified Clay	300	0	80	0	400	0.0	2.0	0.0	3.0
Sewer 7	Vitrified Clay	225	5	80	3	400	0.0	2.0	0.0	3.0
Sewer 8	Masonr y	810	8	500	-	-		-		-

Table 8 Sewers impact assessment summary - end of excavation

Asset	Material	Internal Diamet er (mm)	Maximu m Tensile Strain ( με)	Tensil e Strain Limit (με)	Maximum Compressi ve Strain (με)	Compressi ve Strain Limit (με)	Maximu m Rotation	Rotatio n Limit	Maximu m Pull- out(mm)	Pull- out Limi t (mm
Sewer 1a	Masonr y	1750	149	500	u.	-	-	-	-	-
Sewer 1b	Masonr y	2200	239	500	-	-	-	-		-
Sewer 2a	Masonr y	810	323	500	-	-		-	-	-
Sewer 2b	Masonr y	810	295	500	-	-	-	-	-	-
Sewer 2c	Masonr y	810	483	500	-		-	-	0	-
Sewer 3	Vitrified Clay	300	74	80	26	400	0.0	2.0	0.2	3.0
Sewer 4	Vitrified Clay	300	60	80	18	400	0.0	2.0	0.1	3.0



Asset	Material	Internal Diamet er (mm)	Maximu m Tensile Strain ( με)	Tensil e Strain Limit (με)	Maximum Compressi ve Strain (με)	Compressi ve Strain Limit (με)	Maximu m Rotation	Rotatio n Limit	Maximu m Pull- out(mm)	Pull- out Limi t (mm
Sewer 5	Concret e	300	169	40	20	400	0.0	2.0	0.4	3.0
Sewer 6	Vitrified Clay	300	74	80	2	400	0.3	2.0	1.5	3.0
Sewer 7	Vitrified Clay	225	30	80	108	400	0.0	2.0	0.1	3.0
Sewer 8	Masonr	810	334	500	-	-	-	-	-	

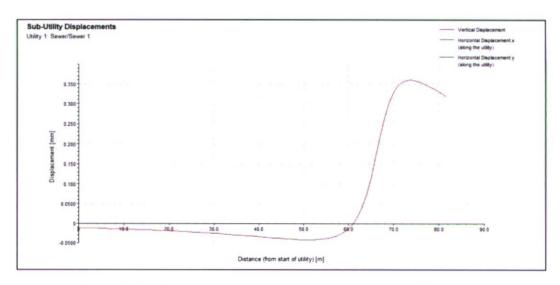


Figure 66 Displacements along sewer 1a induced by demolition works

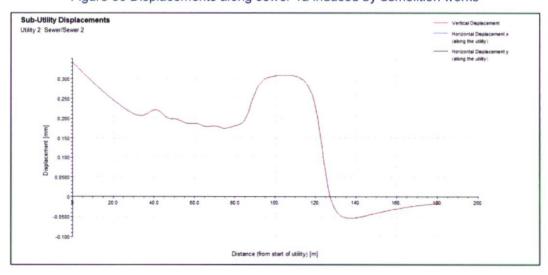


Figure 67 Displacements along sewer 1b induced by demolition works



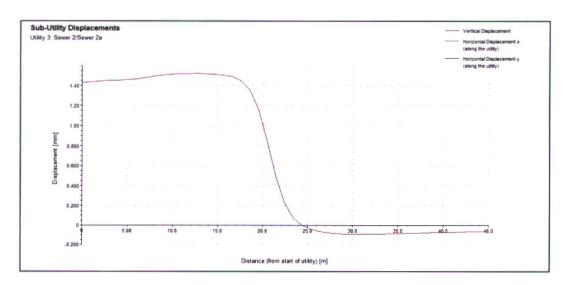


Figure 68 Displacements along sewer 2a induced by demolition works

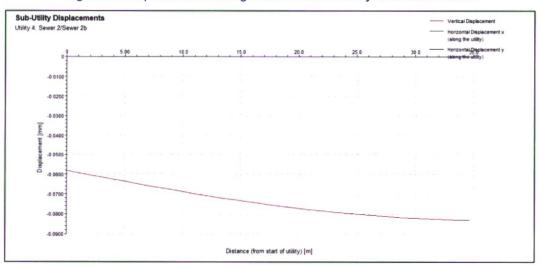


Figure 69 Displacements along sewer 2b induced by demolition works



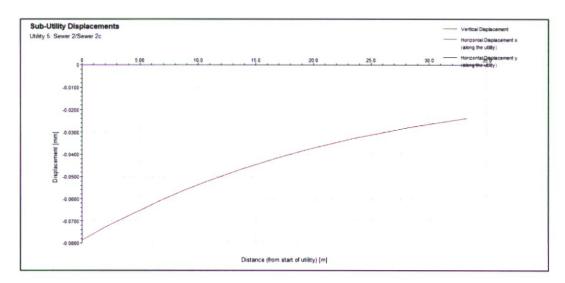


Figure 70 Displacements along sewer 2c induced by demolition works

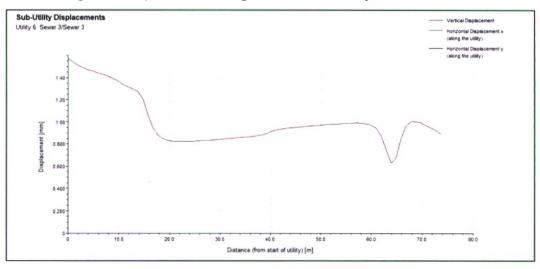


Figure 71 Displacements along sewer 3 induced by demolition works



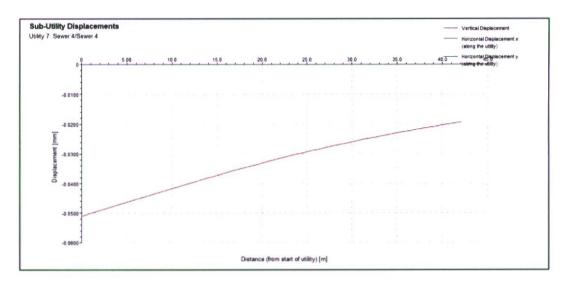


Figure 72 Displacements along sewer 4 induced by demolition works

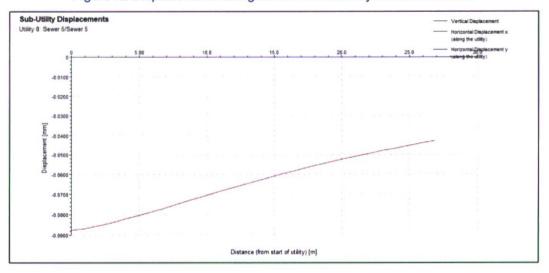


Figure 73 Displacements along sewer 5 induced by demolition works



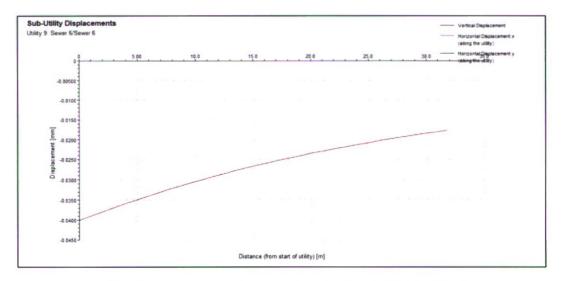


Figure 74 Displacements along sewer 6 induced by demolition works

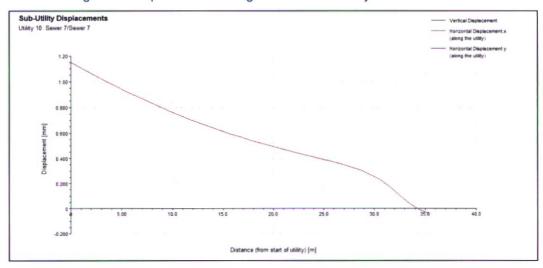


Figure 75 Displacements along sewer 7 induced by demolition works



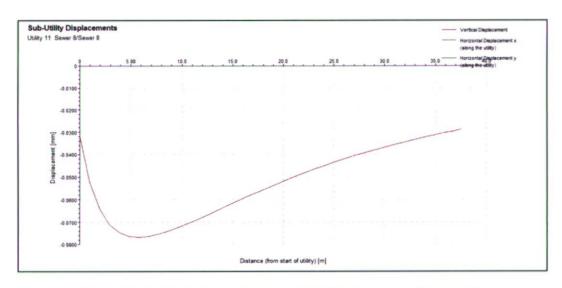


Figure 76 Displacements along sewer 8 induced by demolition works

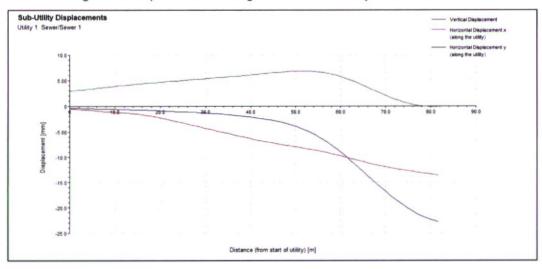


Figure 77 Displacements along sewer 1a induced by walls installation and excavation works



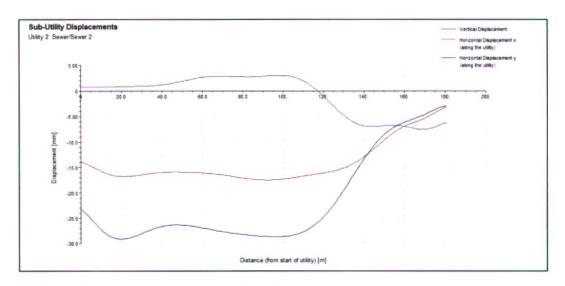


Figure 78 Displacements along sewer 1b induced by walls installation and excavation works

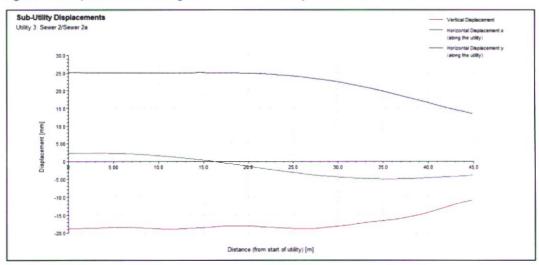


Figure 79 Displacements along sewer 2a induced by walls installation and excavation works



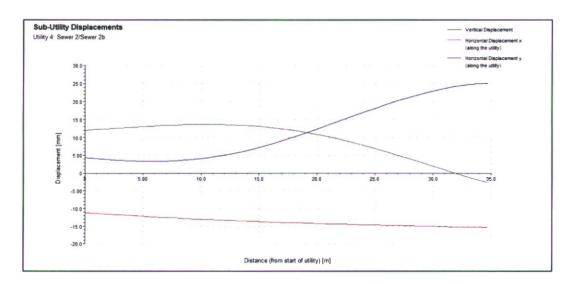


Figure 80 Displacements along sewer 2b induced by walls installation and excavation works

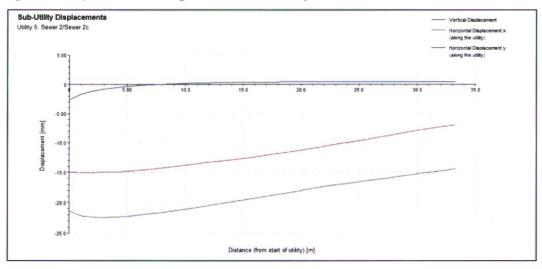


Figure 81 Displacements along sewer 2c induced by walls installation and excavation works



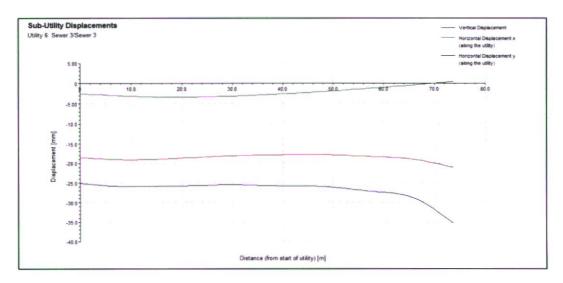


Figure 82 Displacements along sewer 3 induced by walls installation and excavation works

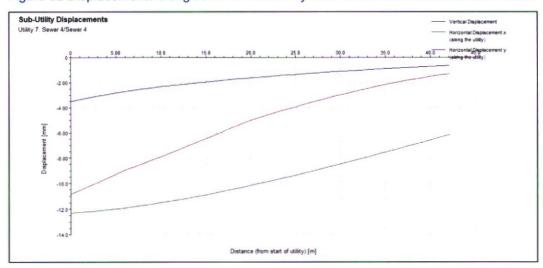


Figure 83 Displacements along sewer 4 induced by walls installation and excavation works



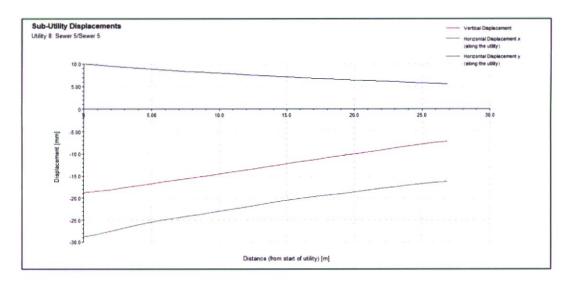


Figure 84 Displacements along sewer 5 induced by walls installation and excavation works

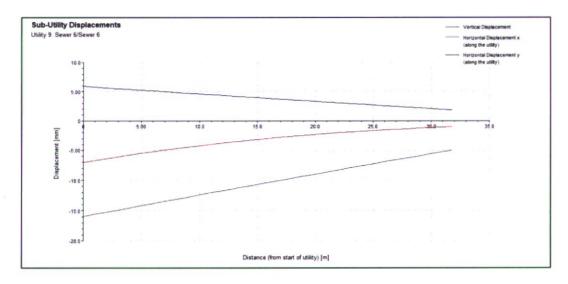


Figure 85 Displacements along sewer 6 induced by walls installation and excavation works



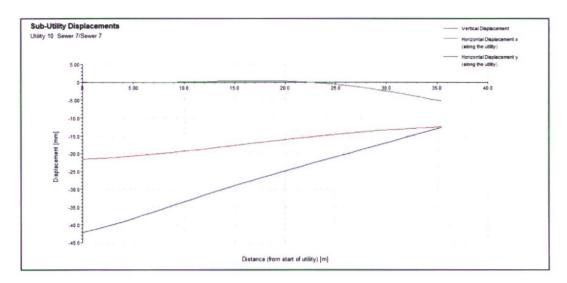


Figure 86 Displacements along sewer 7 induced by walls installation and excavation works

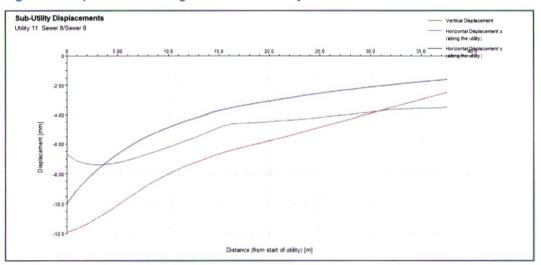


Figure 87 Displacements along sewer 8 induced by walls installation and excavation works



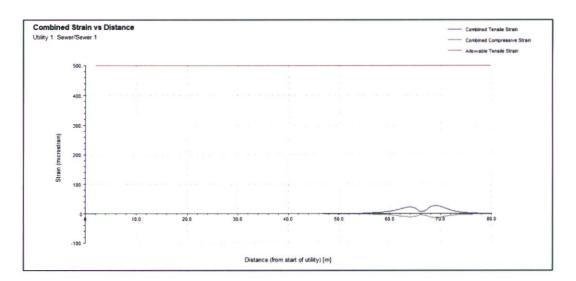


Figure 88 Strains induced in sewer 1a by demolition works

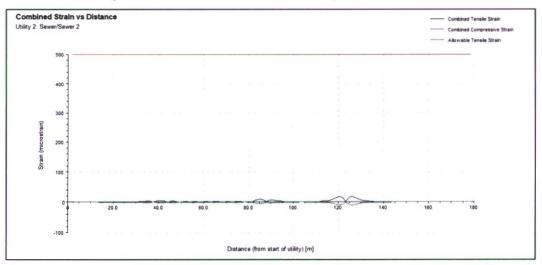


Figure 89 Strains induced in sewer 1b by demolition works



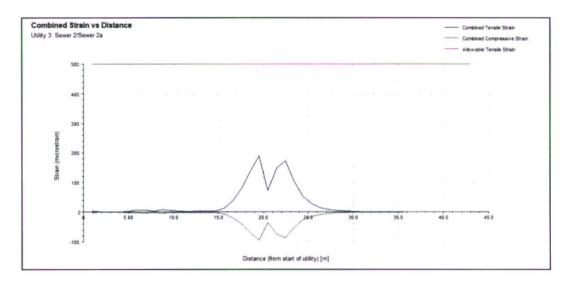


Figure 90 Strains induced in sewer 2a by demolition works

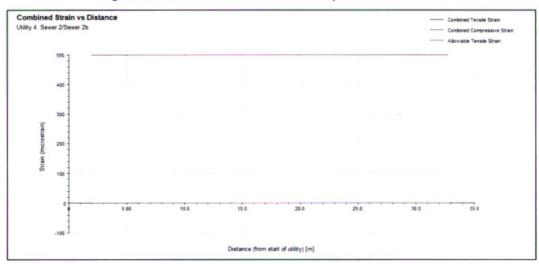


Figure 91 Strains induced in sewer 2b by demolition works



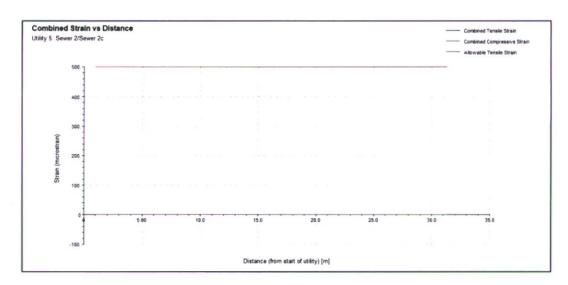


Figure 92 Strains induced in sewer 2c by demolition works

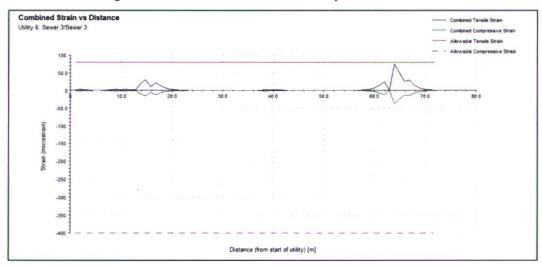


Figure 93 Strains induced in sewer 3 by demolition works



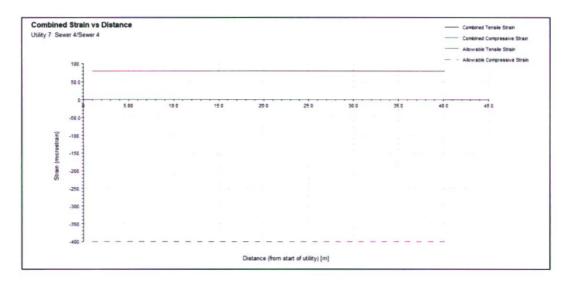


Figure 94 Strains induced in sewer 4 by demolition works

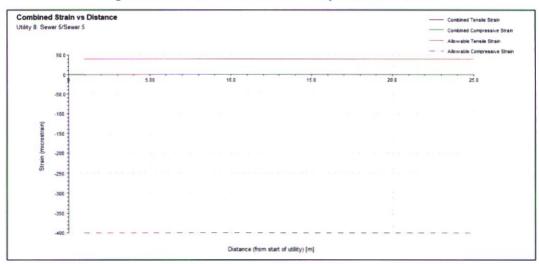


Figure 95 Strains induced in sewer 5 by demolition works



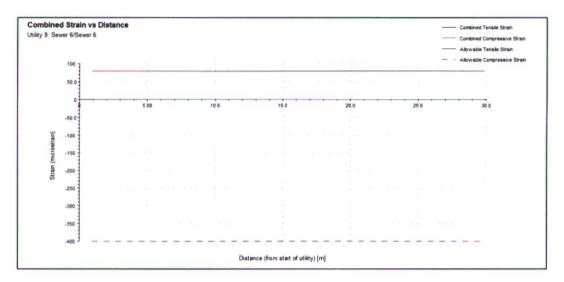


Figure 96 Strains induced in sewer 6 by demolition works

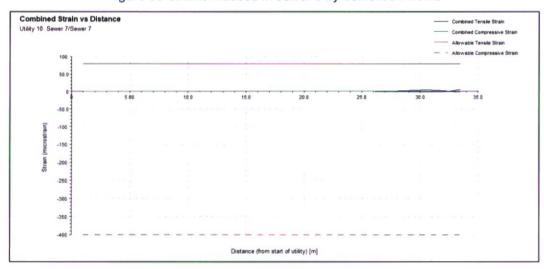


Figure 97 Strains induced in sewer 7 by demolition works



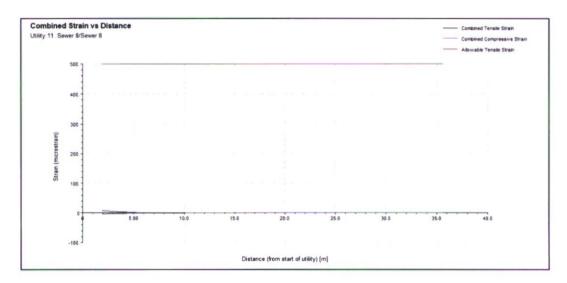


Figure 98 Strains induced in sewer 8 by demolition works

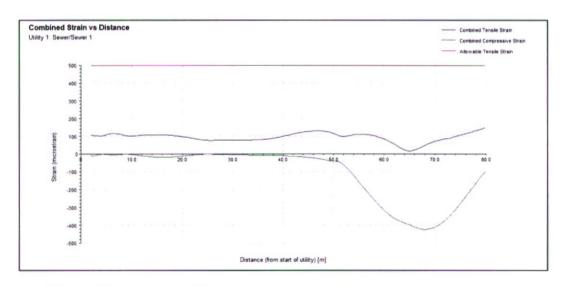


Figure 99 Strains induced in sewer 1a by walls installation and excavation works



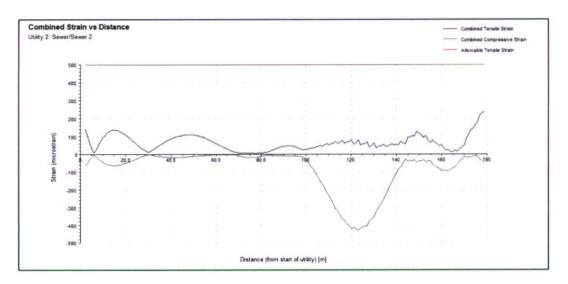


Figure 100 Strains induced in sewer 1b by walls installation and excavation works

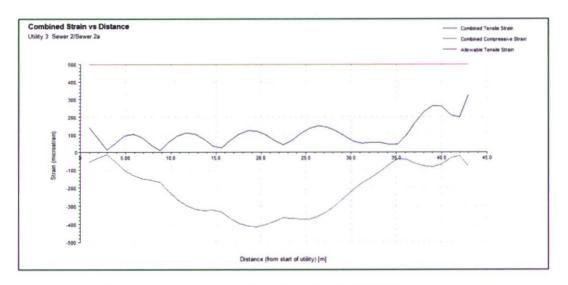


Figure 101 Strains induced in sewer 2a by walls installation and excavation works



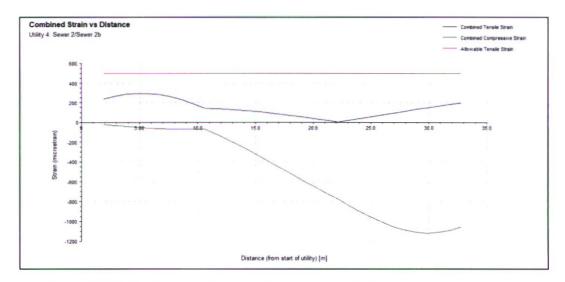


Figure 102 Strains induced in sewer 2b by walls installation and excavation works

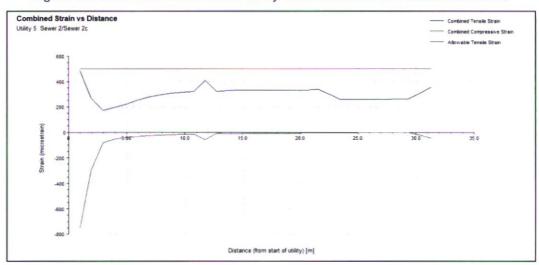


Figure 103 Strains induced in sewer 2c by walls installation and excavation works



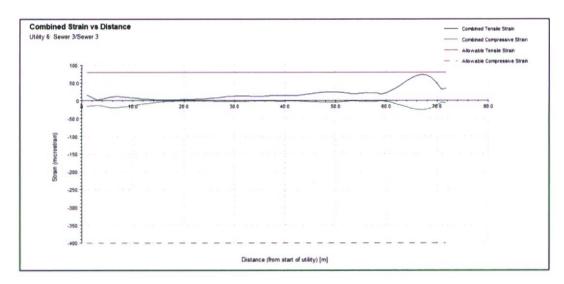


Figure 104 Strains induced in sewer 3 by walls installation and excavation works

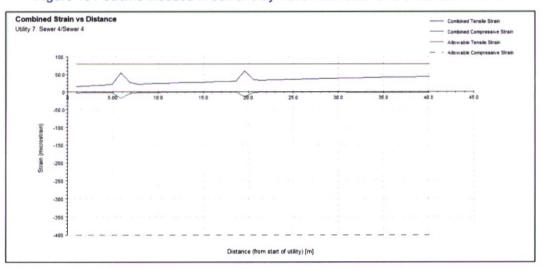


Figure 105 Strains induced in sewer 4 by walls installation and excavation works



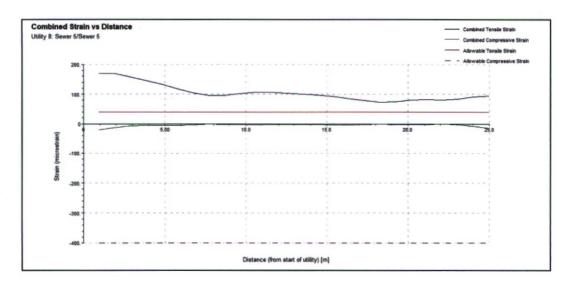


Figure 106 Strains induced in sewer 5 by walls installation and excavation works

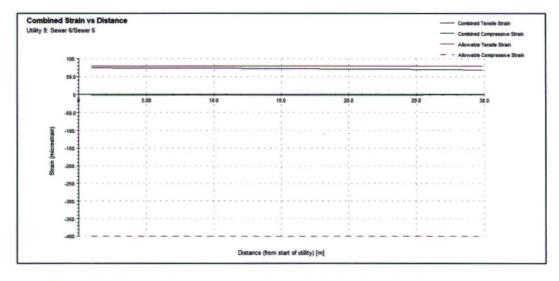


Figure 107 Strains induced in sewer 6 by walls installation and excavation works



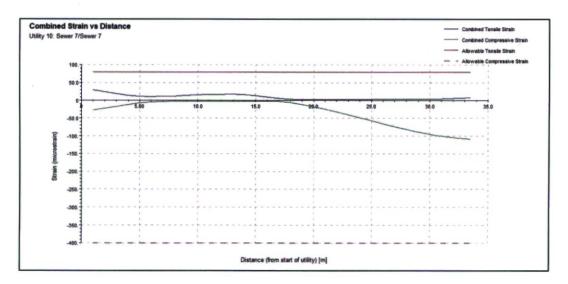


Figure 108 Strains induced in sewer 7 by walls installation and excavation works

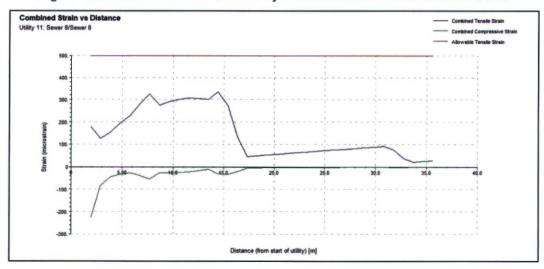


Figure 109 Strains induced in sewer 8 by walls installation and excavation works



#### 7.9 Conclusion

The interaction between the proposed Dublin Central development (Site 2) and the neighbouring properties within the zone of influence of the scheme has been reviewed as part of the GMA study presented herein.

The proposed works primarily involve the demolition of a number of existing buildings present across the Site 2 plot, the excavation of a new one-storey basement across the entire site footprint and the excavation of a deep box, forming the new O'Connell Street Station.

The GMA presented focuses on the temporary work construction stages, primarily comprising existing buildings demolition and basement/box excavation and construction works.

The impact of the various construction stages has been reviewed, evaluating the effects of unloading using Pdisp and simulating the excavation-induced ground movement fields using empirical CIRIA curves in Xdisp. In the latter case, a propped retaining wall solution (during the temporary works stage) has been considered, utilising the CIRIA C760 ground movement curves for excavation in front of high stiffness walls in stiff clay. The curves have been scaled in order to simulate a maximum wall deflection approximately equal to 20mm, as estimated from a number of retaining wall analyses.

In order to best limit ground movements in proximity to movement sensitive neighbouring buildings, due consideration should be given to suitable means and methods of construction.

The results from the GMA analyses are presented in Table 2 (denoting the evaluated damage categorisation in accordance with the Burland criteria described herein). It is observed that the maximum damage classification for the neighbouring properties is *Category 1 – Very Slight*.

Specific wall/façade deflection limits/trigger levels will be developed as part of the proposed monitoring regime (based on the findings presented herein). These will be provided to the design and build contractors with the tender documentation and they will be required to agree and implement a monitoring regime.

It is noted that the predicted ground movements, the associated wall tensile strains, and the level of damage categorisation, are considered to be moderately conservative in view of the relatively cautious data selection and *greenfield* nature of the assessment undertaken.

The interaction between the proposed Dublin Central development (Site 2) and the LUAS light railway within the zone of influence of the scheme has been reviewed as part of the GMA study.

The results from the GMA analysis are presented in Table 3 and Table 4. The maximum track settlement at any point is approximately 12.5mm. The maximum calculated cant and twist of the track during the proposed works is 0.5mm (cant) and 0.1mm (twist at 3m spacing).

The predicted ground movements and the associated LUAS light rail track movements are considered to be moderately conservative, in view of the relatively cautious data selection and *greenfield* nature of the assessment undertaken.

The predicted movement results for the Luas light rail tracks do not show any onerous conditions for the assets and the calculated movements are below the limits proposed by the Code of engineering practice for works on, near, or adjacent the Luas light rail system. Therefore, the proposed Site 2 works do not highlight any concerns regarding the day-to-day operations.



In view of the above findings, the risk of impact on the LUAS light railway associated with the proposed development construction, is considered to be low.

It is concluded that the sewers in proximity of the site will be subject to movements and strains within the allowable criteria (generally based on experience of similar projects) except for Sewer 5, for which the estimated tensile strain exceeds the allowable limit.

In view of the above, with the exception of Sewer 5, the risk to the utilities in proximity to the development, due to the ground movements induced by the proposed scheme construction, is considered to be low.

The assessment presented herein is dependent and reliant on the works being undertaken by an experienced contractor, high quality workmanship, and appropriate supervision of construction means and methods by experienced personnel. In addition, the assessment is based on the assumption that the buildings structures surrounding the proposed development footprint are in a good state.

It is recommended that this report is reviewed and understood in full by the project team and major stakeholders. Where significant changes are made to items such as construction sequencing, temporary propping arrangements and scheme design the engineer should thoroughly review the discrepancy and evaluate any potential impacts on ground movement and building damage. If necessary, the building damage categories should be re-evaluated.

It should be noted that the findings of this report shall be revisited in detailed design stage to include, any significant change in the proposed construction sequence, and/or update structural loads according to the final proposed foundation layout.

It is critical that the permanent and temporary works designs are carried out in a coordinated manner between performance specified elements and substructure contractors, with the aim to ensure that such design elements are in alignment with the assumptions/findings of the GMA and overall design intent.



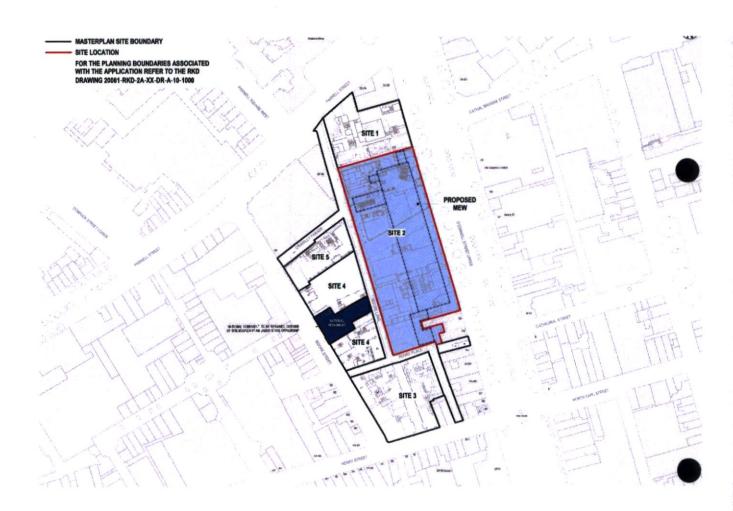
## 8. References

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- 6. Plaxis 2D CE V21.00:0 Manual, September 2020.
- 7. Tomlinson, M.J. (2001). Foundation Design and Construction, Pearson Prentice Hall, Harlow England.



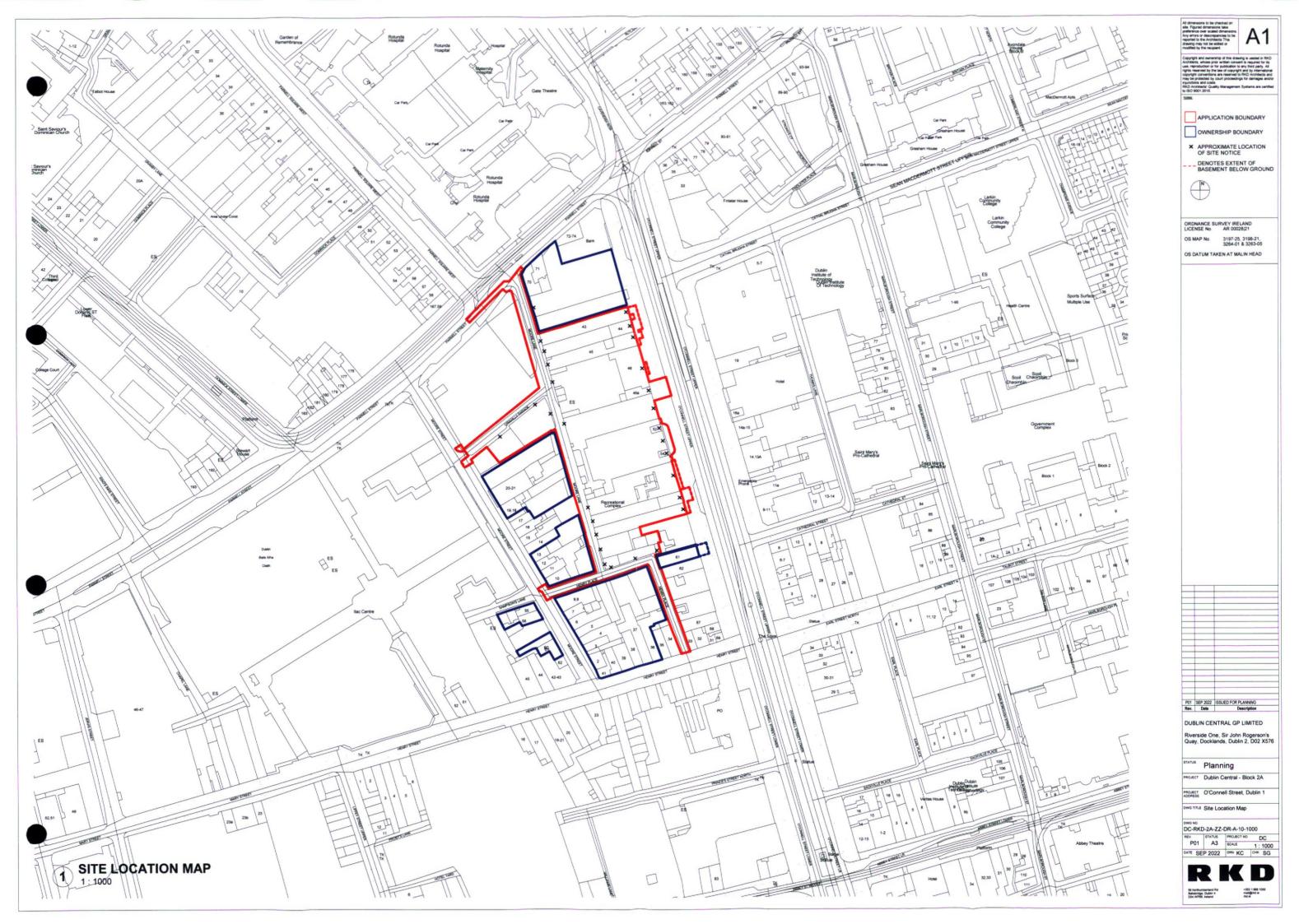
# **APPENDICES**

# A. Overall Development Boundary and Existing Structures

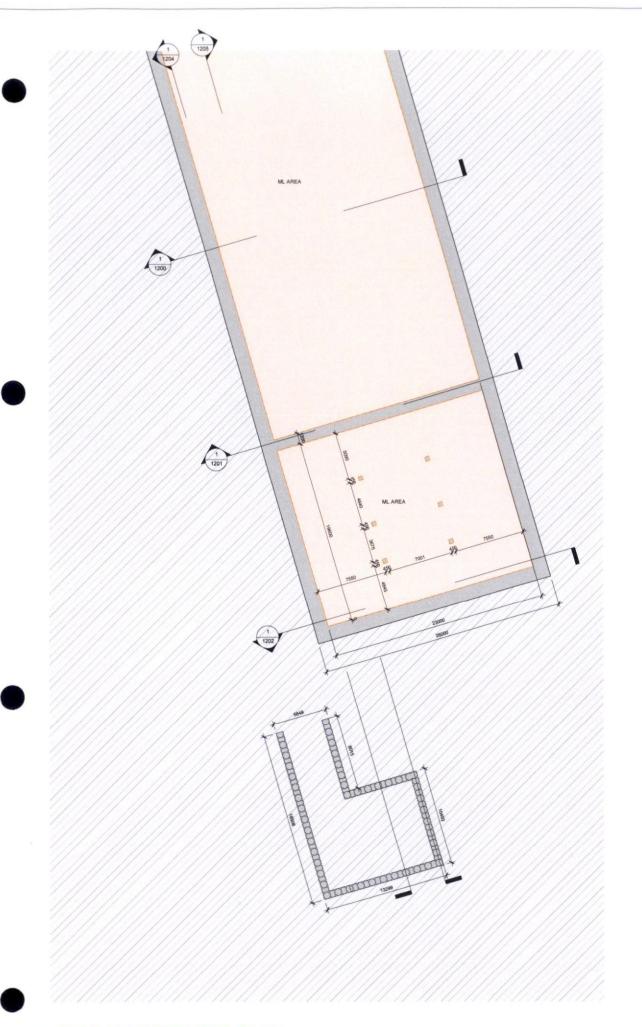




B. Site 2 Architectural Drawings







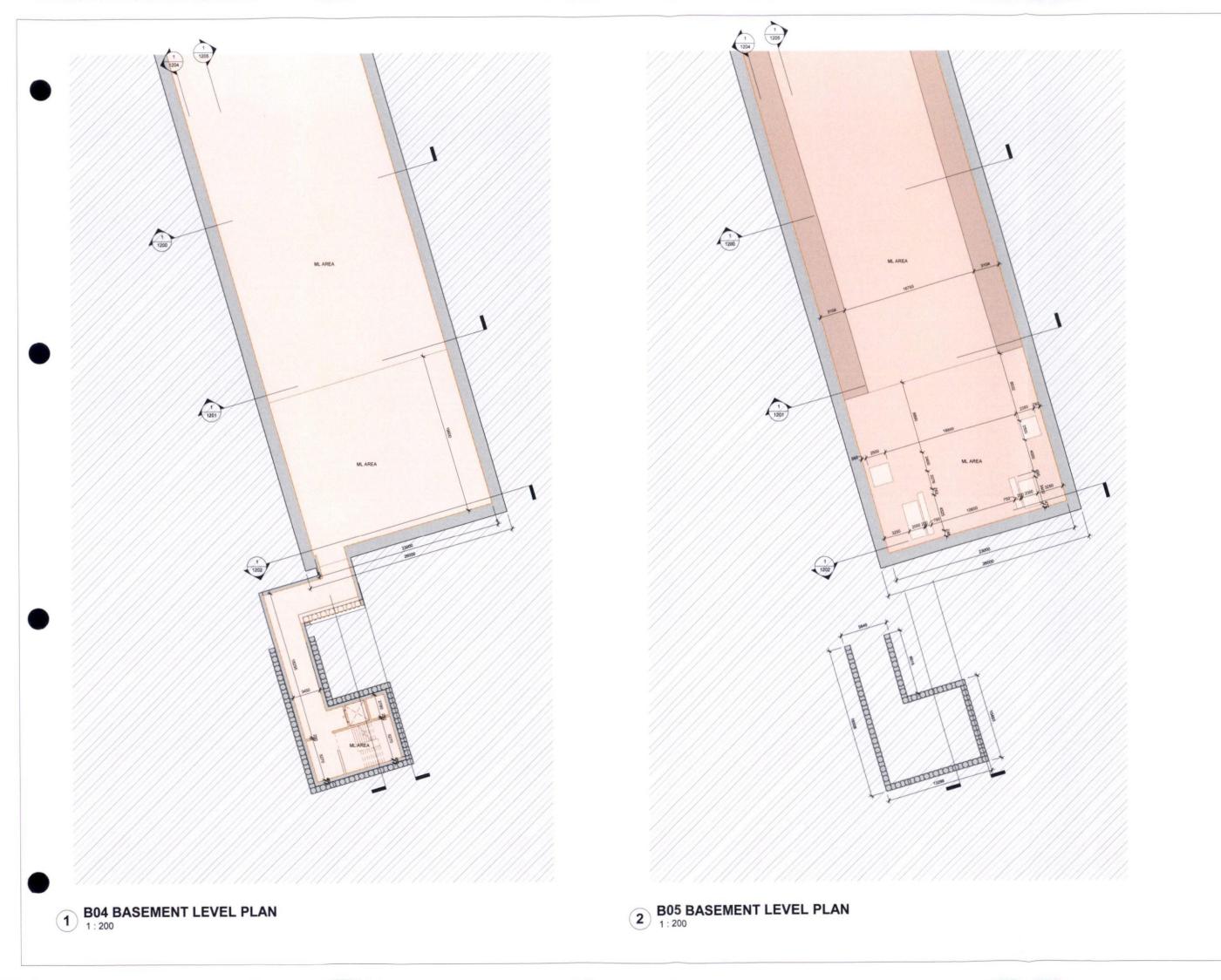
1 B06 BASEMENT LEVEL PLAN



Planning

PROJECT O'Connell Street, Dublin 1





All dimensions to be checked or site. Figured dimensions take preference over scaled dimensions Any errors or descrepancies to I reported to the Architects. This drawing may not be edited or

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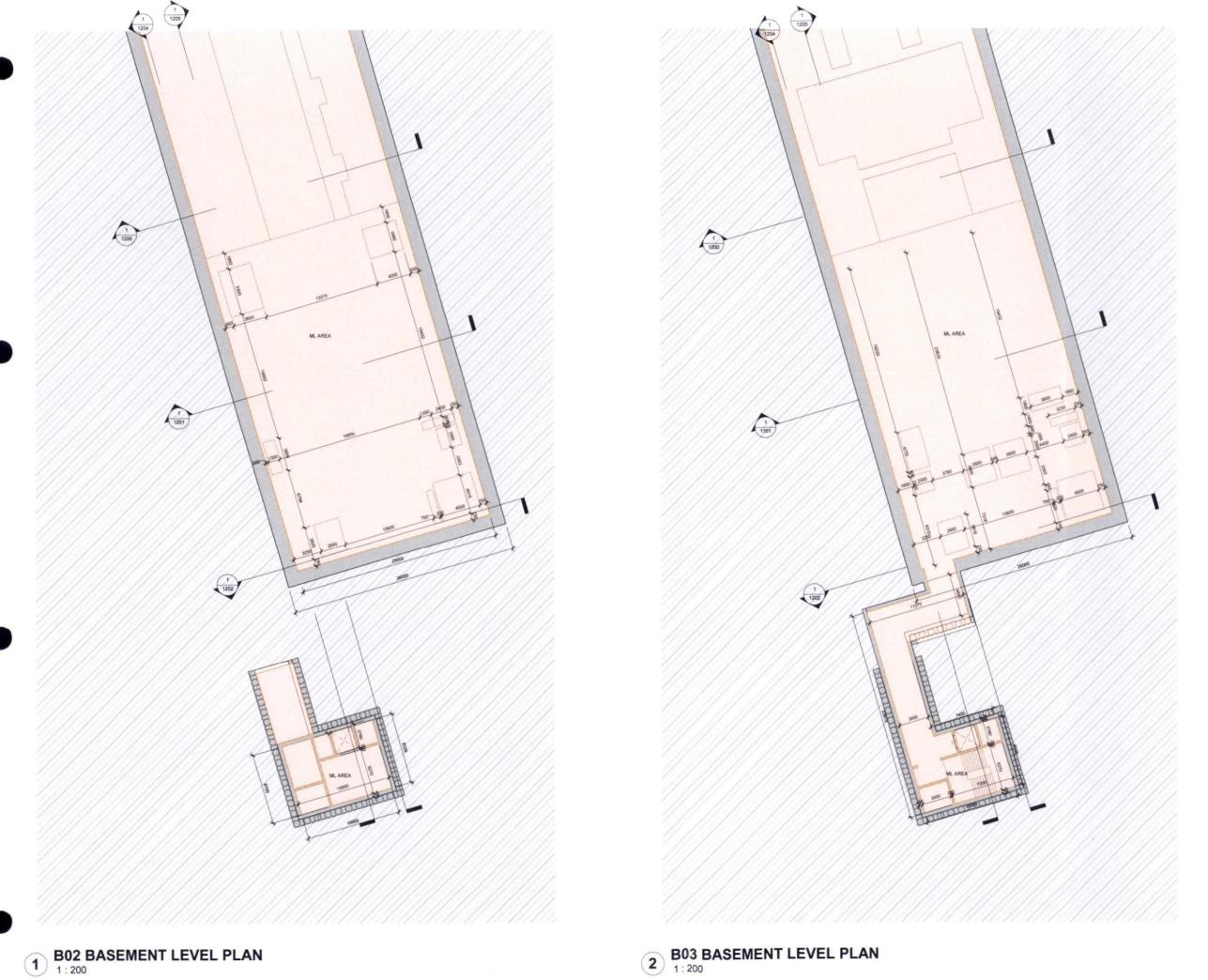
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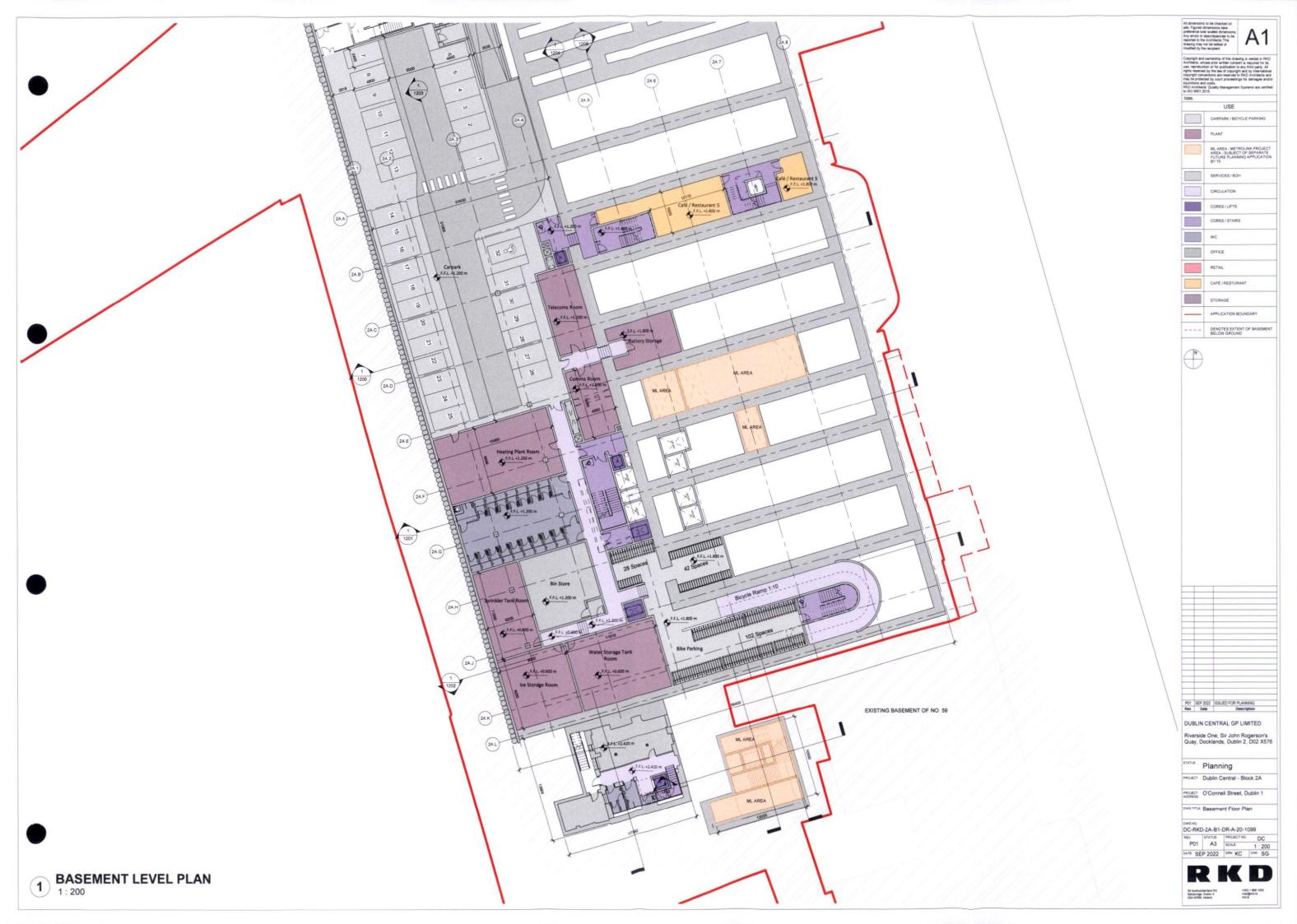
PO1 A3 SCALE ...

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Planning

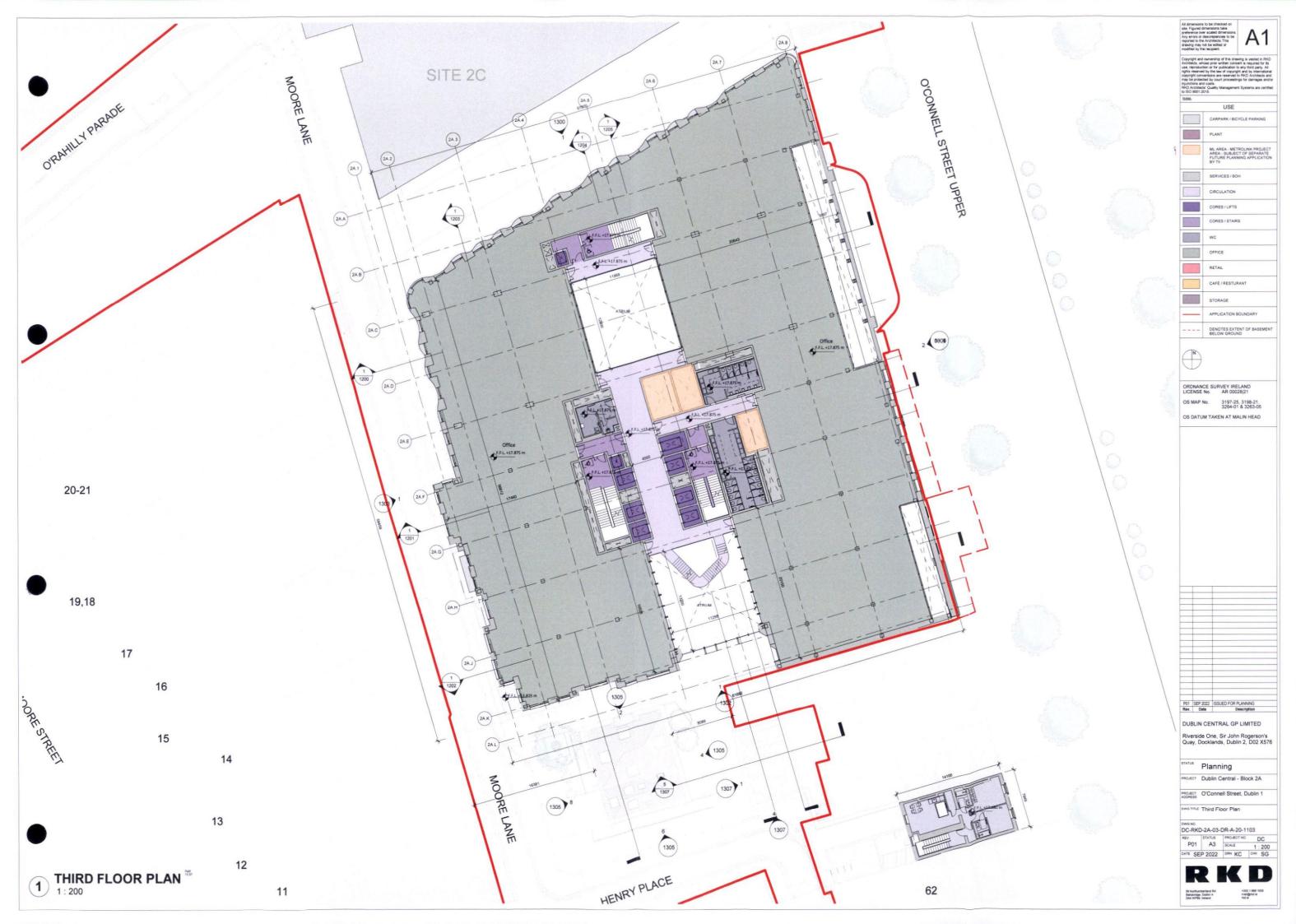
Dublin Central - Block 2A

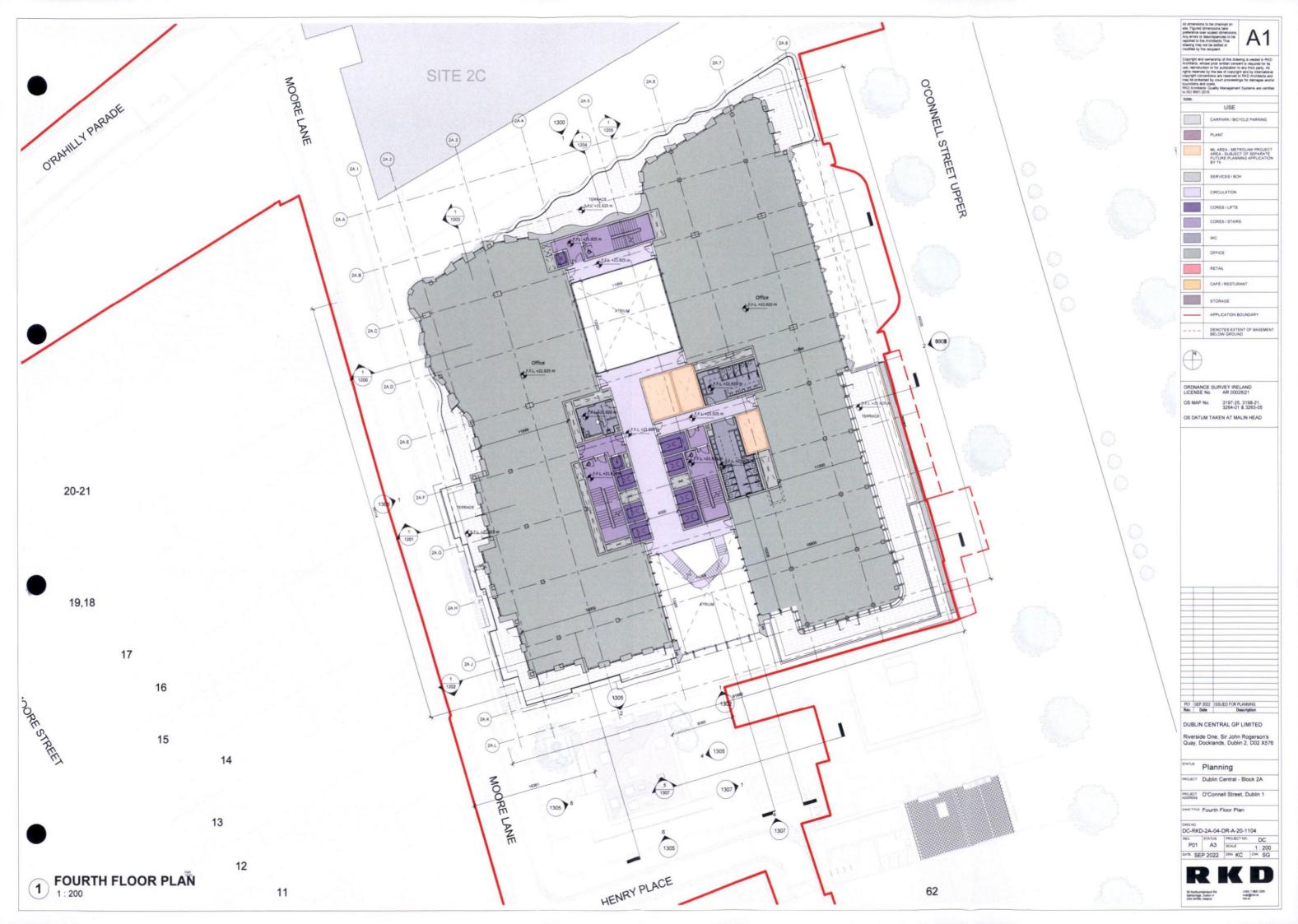






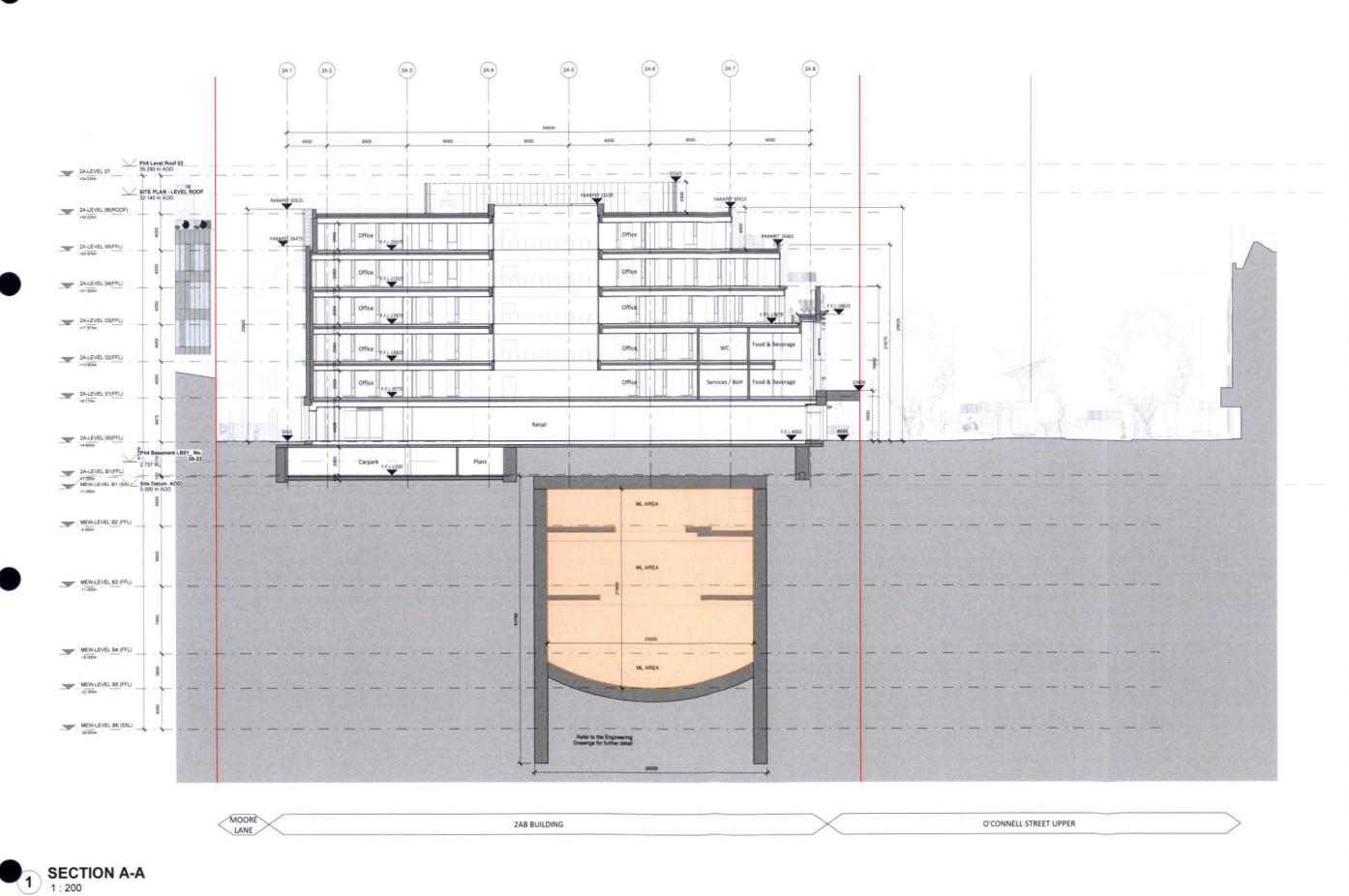




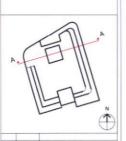








REFER TO ARCHITECTURAL ELEVATIONS (DRAWING NO.'S A1300-A1307) AND ARCHITECT'S DESIGN STATEMENT FOR MATERIAL FINISHES.



DUBLIN CENTRAL GP LIMITED

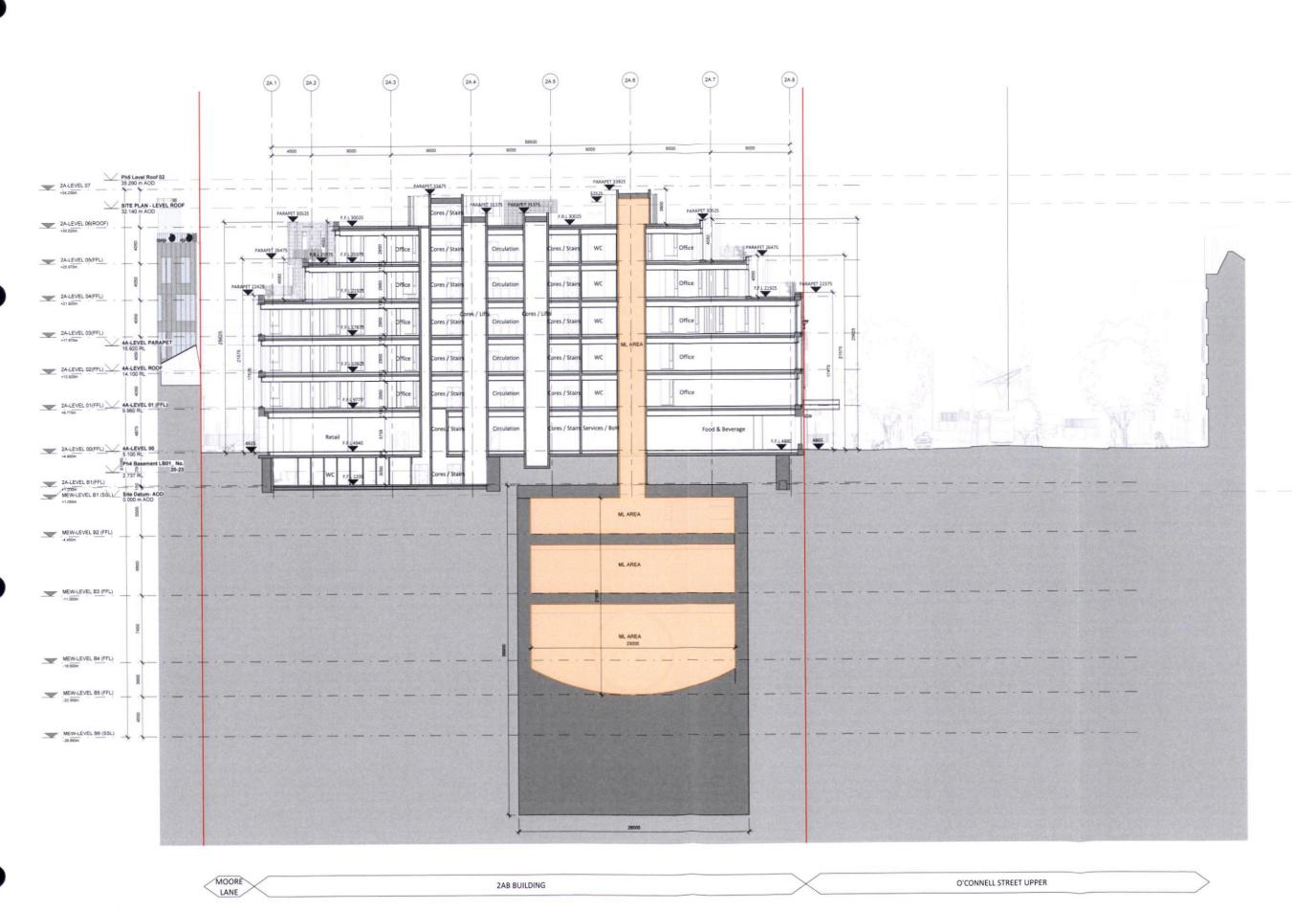
STATUS Planning

PROJECT Dublin Central - Block 2A

PROJECT O'Connell Street, Dublin 1

DWG TITLE Section A-A

DC-RKD-2A-ZZ-DR-A-30-1200



All dimensions to be checked on site. Figured dimensions take preference over scaled dimensions. Any errors or descrepancies to be reported to the Architects This drawing may not be edited or

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Notes.

USE

ML Area - MetroLink Project Subject of Separate Future Planning Application by TII APPLICATION BOUNDARY

REFER TO ARCHITECTURAL ELEVATIONS (DRAWING NO.'S A1300-A1307) AND ARCHITECT'S DESIGN STATEMENT FOR MATERIAL FINISHES.

B

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Date Description

DUBLIN CENTRAL GP LIMITED

Riverside One, Sir John Rogerson's
Quay, Docklands, Dublin 2, D02 X576

Planning

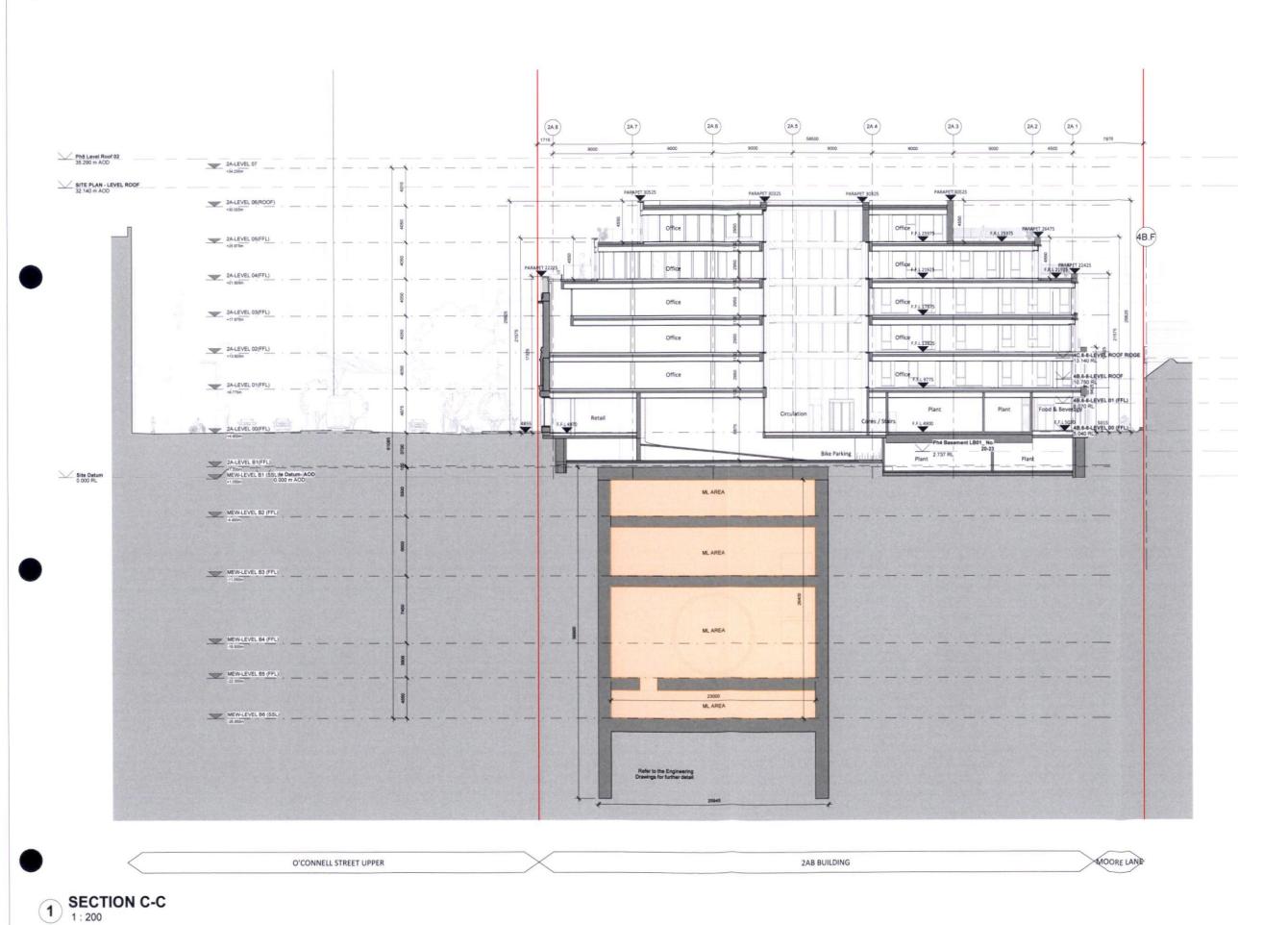
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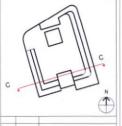
Section B

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1 SECTION B-B



ML Area - MetroLink Project Area -Subject of Separate Future Planning Application by TII APPLICATION BOUNDARY



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DUBLIN CENTRAL GP LIMITED Riverside One, Sir John Rogerson's Quay, Docklands, Dublin 2, D02 X576

STATUS Planning

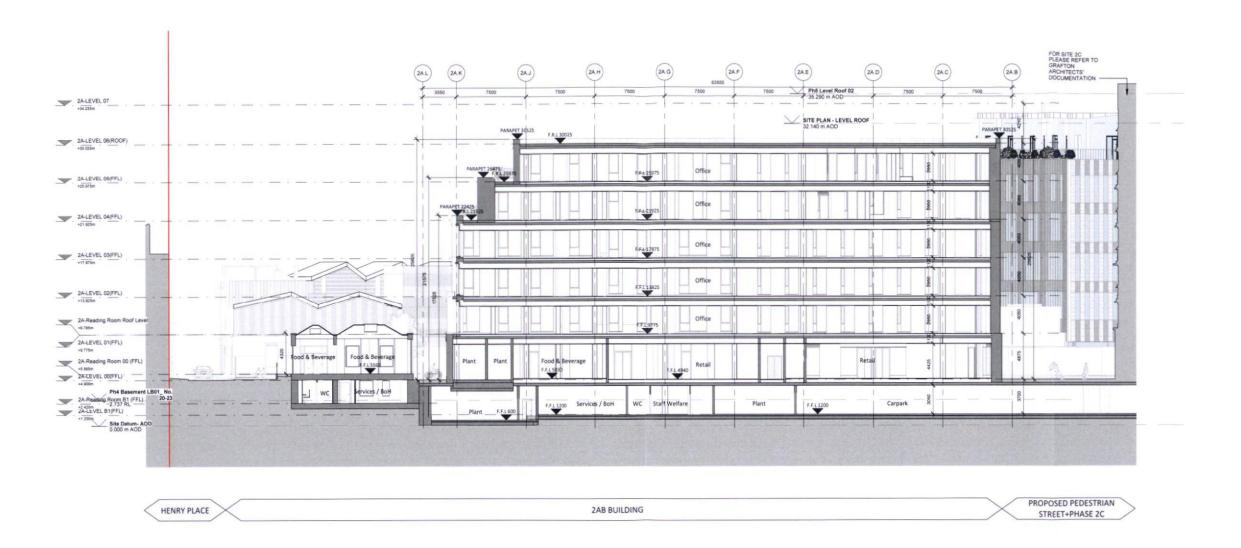
PROJECT Dublin Central - Block 2A

PROJECT O'Connell Street, Dublin 1

DWG TITLE Section C-C

| DUC-RKD-2A-ZZ-DR-A-30-1202 | REV. | STATUS | PROJECT NO. | DC | SCALE | 1 : 200 | DATE | SEP 2022 | DRN | KC | CHM | SG





1 SECTION D-D

ML Area - MetroLink Project Area -Subject of Separate Future Planning Application by TII APPLICATION BOUNDARY DUBLIN CENTRAL GP LIMITED STATUS Planning PROJECT Dublin Central - Block 2A PROJECT O'Connell Street, Dublin 1 DWG TITLE Section D-D

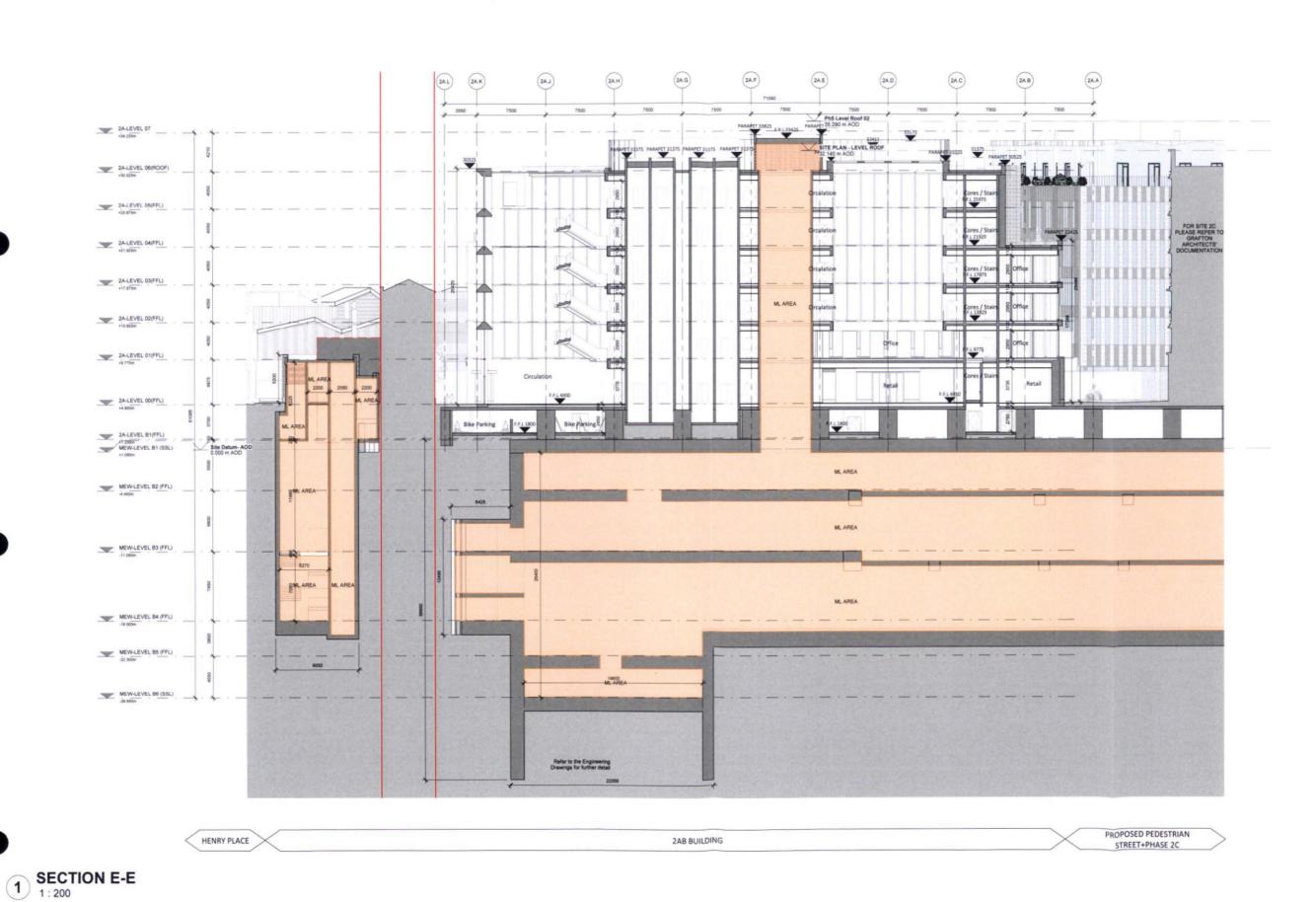
P01 SEP 2022 ISSUED FOR PLANNING Rev. Date Description

Riverside One, Sir John Rogerson's Quay, Docklands, Dublin 2, D02 X576

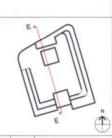
| DC-RKD-2A-ZZ-DR-A-30-1203 | REV | STATUS | PROJECT NO | DC | SCALE | 1 : 200 | DATE | SEP 2022 | DRN KC | CHR | SG



50 Northumberland Rd Baltsbridge, Dublin # DG4 WPB9, Ireland



ML Area - MetroLink Project Area -Subject of Separate Future Planning Application by Til APPLICATION BOUNDARY



P01 SEP 2022 ISSUED FOR PLANNING
Rev. Date Description DUBLIN CENTRAL GP LIMITED

Riverside One, Sir John Rogerson's Quay, Docklands, Dublin 2, D02 X576

Planning Planning

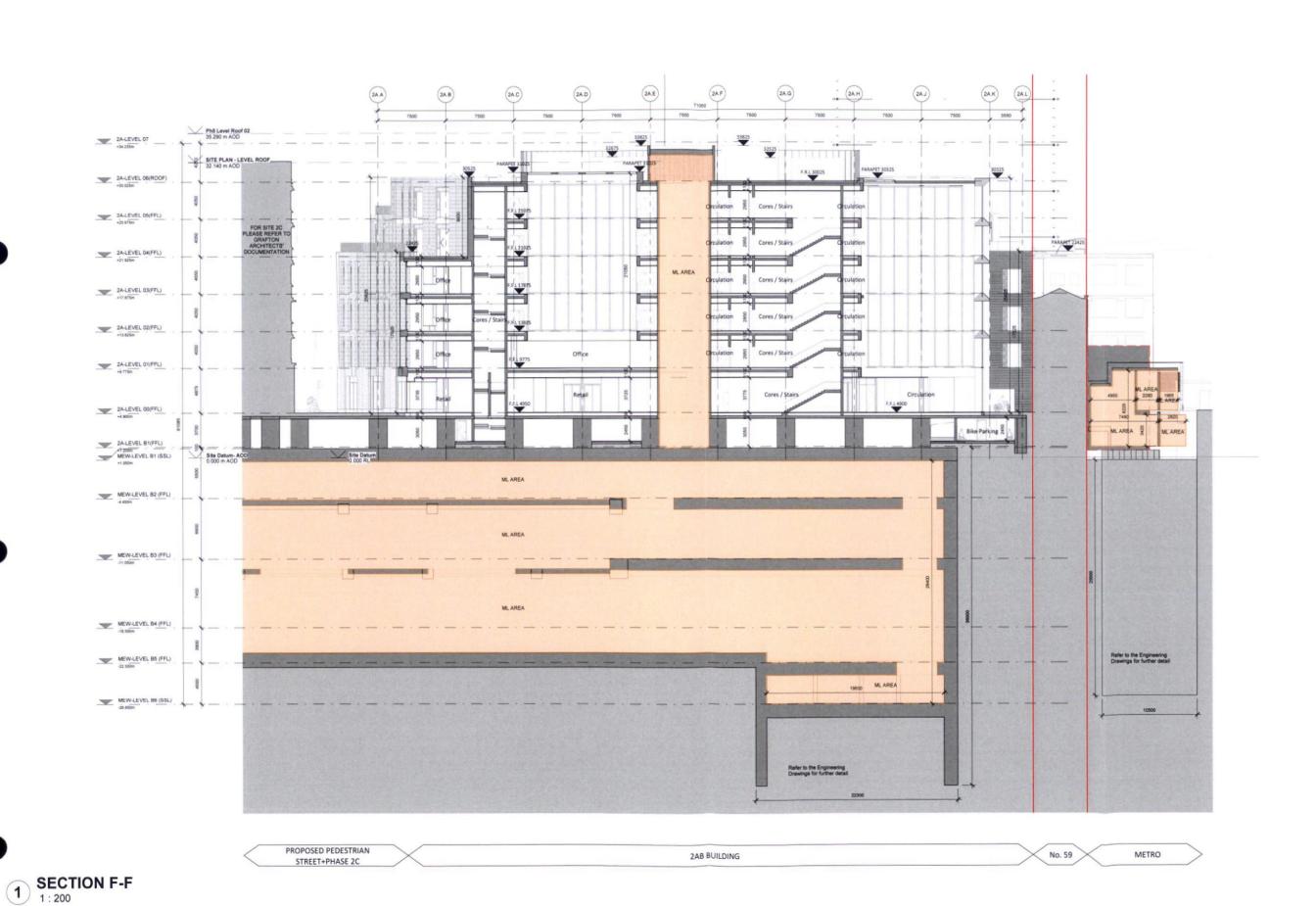
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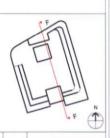
PROJECT O'Connell Street, Dublin 1

DWG TITLE Section E-E

DC-RKD-2A-ZZ-DR-A-30-1204







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DUBLIN CENTRAL GP LIMITED

STATUS Planning

PROJECT Dublin Central - Block 2A

PROJECT O'Connell Street, Dublin 1

DWG TITLE Section F-F

| DC-RKD-2A-ZZ-DR-A-30-1205 | | PROJECT NO | DC | RBV | RBV | A3 | RGALE | 1 | 200 | DATE SEP 2022 | DRN KC | CMK SG

RKD

59 Northumberland Rd Ballsbridge, Dublin 4 004 WP69, Ireland

KEY MATERIAL SITE 2A/B

10 GLAZING

ur. Iszing on NE / NW corners.

2AB.12 Upper Floors - Metal Framed Glazing - Dark Colour.

2AB.15 Frameless glazed balustrade.

2AB.16 Metal Balustrade - light grey colour

20 SOUTH EAST FACADE

2AB.23 Brick openings / louvre for air extract

30 CARLTON

2AB.30 Re-instated Canopies - Light Colour.

2AB.21 Perforated Metal Screen - Bronce Colour 2AB.31 Retained Carlton Facade - Light Colour.

2AB.32 Stone Pilasters - Light Colour

40 NORTH FACADE

2AB.40 Triangular Profiled Recon-Stone - Light
Colour (Type A).
Colour (Ty

\*Finishes vary based on existing plots.

50 SOUTH WEST FACADE

2AB.52 Two common brick textures and two unique brick textures per bay - Various Grey Colours. (Type C)

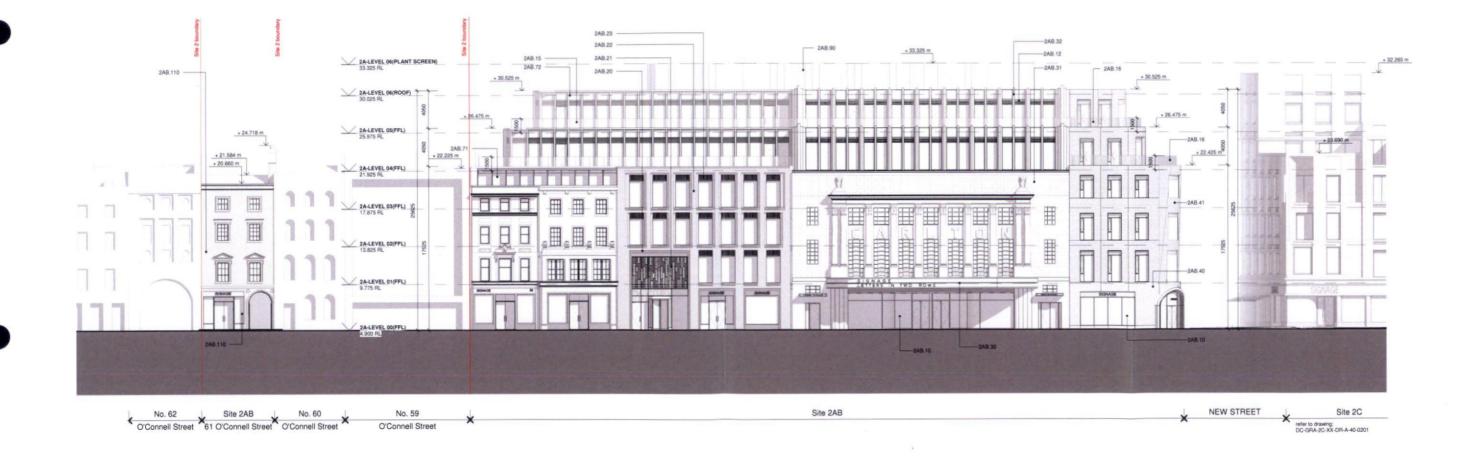
OTHERS

2AB.60 Dark Grey Recon-Metal Frame with Bronze Coloured Metal Reveals.

2AB.71 Clearstory - Glazing Above Parapet.

2AB.72 Upper Floor - Metal Cladding - Dark grey Colour.

2AB.90 Metal plant screen - Light grey Colour 2AB.100 Reading Room Refurbishment



ports
Codes refer to the System Reference Sheet and
unchitectural Specification
Il details are indicative and are subject to confirmatio
y main contractor's chosen specialist subcontractors
ord simplicity.

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P01 - PLANNING APPLICATION

November 2021 PLANNING APPLICATION XX.XX.2021 FOR INFORMATION date issue description



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DUBLIN CENTRAL - SITE 2B 186 number **ELEVATION EAST** 1:200 @ A1 (1:200@A3) PLANNING APPLICATION SEP 2022 MGB drawing no revision DC-ACM-2AB-ZZ-DR-A-40-1310 P01 KEY MATERIAL SITE 2A/B

2AB.10 Retail Base - Metal Framed Glazing Shop Fronts - Dark Colout - Curved glazing on NE / NW corners + Carlton

\*Curved glazing on NE / NW corners. 2AB.12 Upper Floors - Metal Framed Glazing - Dark Colour.

2AB.15 Frameless glazed balustrade.

2AB.16 Metal Balustrade - light grey colour

20 SOUTH EAST FACADE

2AB.20 Reckon Stone

2AB.23 Brick openings / louvre for air extract

30 CARLTON

2AB.32 Stone Pilasters - Light Colour

2AB.21 Perforated Metal Screen - Bronce Colour 2AB.31 Retained Carlton Facade - Light Colour.

2AB.41 Flat / Wavy Brick - Red Colour. (Type B)
"Feature stepping out on NE corner

50 SOUTH WEST FACADE 40 NORTH FACADE

2AB.40 Triangular Profiled Recon-Stone - Light
Colour (Type A).
Colour (Ty

2AB.51 Two common brick textures and two unique brick textures per bay - Various red Colours. (Type C)

2AB.52 Two common brick textures and two unique brick textures per bay - Various Grey Colours. (Type C)

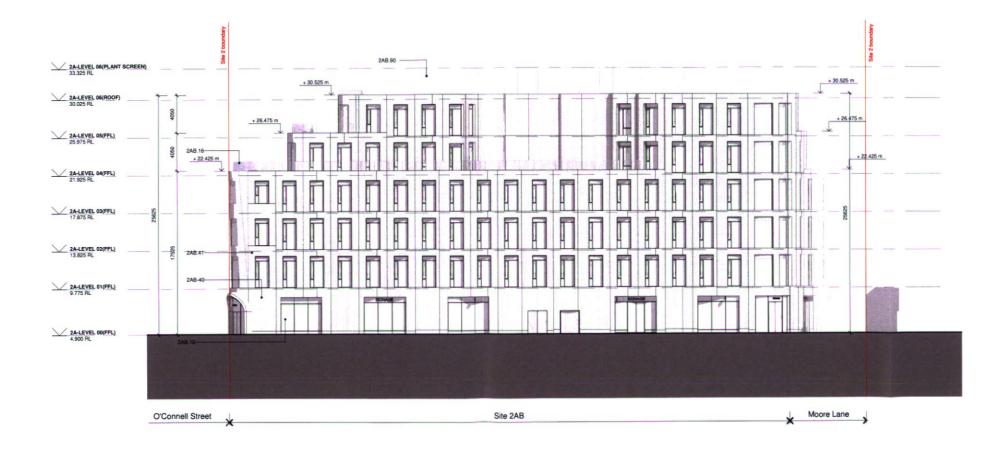
\*Finishes vary based on existing plots.

**OTHERS** 

2AB.60 Dark Grey Recon-Metal Frame with Bronze Coloured Metal Reveals.

2AB.72 Upper Floor - Metal Cladding - Dark grey Colour.

2AB.90 Metal plant screen - Light grey Colour. 2AB.100 Reading Room Refurbishment



This drawing is to be read in conjunction with all relevant architect's and engineers' drawings, specifications and reports.

Codes refer to the System Reference Sheet and Architectural Specification.

All details are indicative and are subject to confirmation by main contractor's chosen specialist subcontractors and suppliers.

Any landscaping shown is indicative, for details of public realm design refer to Landscape Architect's information For existing and proposed statemal levels refer to Landscape Architect's drawings.

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P01 - PLANNING APPLICATION revision schedule

 P01
 November 2021
 PLANNING APPLICATION

 P00
 XXXXX.2021
 FOR INFORMATION

 rev
 date
 issue description



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DUBLIN CENTRAL - SITE 2B 186 **ELEVATION NORTH** 1:200 @ A1 (1:400@A3) S2: SUITABILITY FOR INFORMATION SEP 2022 drawn by MGB checked JD DC-ACM-2AB-ZZ-DR-A-40-1311 P01 KEY MATERIAL SITE 2A/B

plazing on NE / NW corners.

2AB.12 Upper Floors - Metal Framed Glazing - Dark Colour.

2AB.15 Frameiess glazed balustrade.

2AB.16 Metal Balustrade - light grey colour

20 SOUTH EAST FACADE

30 CARLTON

2AB.21 Perforated Metal Screen - Bronce Colour 2AB.31 Retained Carlton Facade - Light Colour. 2AB.32 Stone Pilasters - Light Colour

40 NORTH FACADE 50 SOUTH WEST FACADE

2AB.40 Triangular Profiled Recon-Stone - Light Colour (Type A).

Colour (Type A).

Cverial shape to follow flat / wavy cladding above.

Feature curved archeay on NE comer \*
\*Except above to be metal clear / profiled to metal c

match adjacent floads.

2AB.51 Two common brick testures and two unique brick testures per bay - Various red

Feature stepping out on NE corner

Colours, (Type C)

\*Finishes vary based on existing plots.

OTHERS

2AB.60 Dark Grey Recon-Metal Frame with Bronze Coloured Metal Reveals.

2AB.71 Clearstory - Glazing Above Parapet. 2AB.72 Upper Floor - Metal Cladding - Dark grey Colour.

2AB.90 Metal plant screen - Light grey Colour 2AB.100 Reading Room Refurbishment

+ 36.345 m + 33.325 m 5-5 2AB.12-2A-LEVEL 06(PLANT SCREEN) 33.325 RL + 30.525 m 2A-LEVEL 06(ROOF) 30.025 RL -2A-LEVEL 05(FFL) 25.975 RL -2A-LEVEL 04(FFL) 21.925 RL 2A-LEVEL 03(FFL) 17.875 RL -+ 16.075 m 2A-LEVEL 02(FFL) 13.825 RL + 11.780 m + 10.980 m 2A-LEVEL 01(FFL) 9.775 RL 2A-LEVEL 00(FFL)

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Site 2C

refer to drawing: DC-GRA-2C-XX-DR-A-40-0202

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**NEW STREET** 

in abeyance schedule

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Site 2AB

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**DUBLIN CENTRAL - SITE 2B ELEVATION WEST** 1:200 @ A1 (1:200@A3) PLANNING APPLICATION SEP 2022 drawing no revision

DC-ACM-2AB-ZZ-DR-A-40-1312 P01